

**ENCE 353 Final Exam, Open Notes and Open Book**

Name : \_\_\_\_\_

**Exam Format and Grading.** This take home final exam is open notes and open book. You need to comply with the university regulations for academic integrity.

Answer Question 1. Then answer **three of the five** remaining questions. Partial credit will be given for partially correct answers, so please show all your working.

**IMPORTANT:** Only the **first four questions** that you answer will be graded, so please **cross out the two questions you do not want graded** in the table below. Also, before submitting your exam, check that **every page has been scanned correctly**.

Question	Points	Score
1	20	
2	10	
3	10	
4	10	
5	10	
6	10	
Total	50	

**Question 1: 20 points**

**COMPULSORY: Moment-Area, Virtual Forces, Superposition.** Figure 1 is a front elevation view of a simple beam structure carrying two external loads  $P$ . The beam has section properties  $EI$  along its entire length.

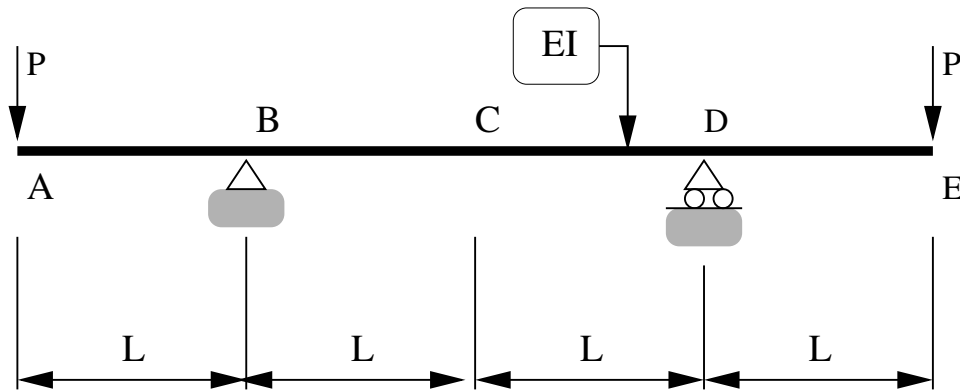


Figure 1: Simple beam structure with symmetric loads  $P$ .

**[1a]** (3 pts) Use the method of **moment area** to show that the beam rotation at  $D$  (measured clockwise) is:

$$\theta_D = \frac{PL^2}{EI}. \quad (1)$$

**[1b]** (3 pts) Use the method of **moment area** to show that the beam deflection at  $C$  (measured upwards) is:

$$\Delta_C = \frac{1}{2} \frac{PL^3}{EI}. \quad (2)$$

[1c] (3 pts) Use the method of **moment area** to show that the that the beam deflection at E (measured downwards) is:

$$\Delta_E = \frac{4}{3} \frac{PL^3}{EI}. \quad (3)$$

[1d] (4 pts) A function is said to be even if it has the property  $f(x) = f(-x)$  (i.e., it is symmetric about the y axis). And a function is said to be odd if it has the property  $g(x) = -g(-x)$  (i.e., it is skew-symmetric about the y axis). One example of an even function is  $\cos(x)$ , and one example of an odd function is  $\sin(x)$ .

Using high-school-level calculus (or otherwise), show that:

$$\int_{-h}^h f(x)g(x)dx = 0. \quad (4)$$

Please show all of your working.

Figure 2 shows the same beam structure, but now the external loads are rearranged so that one load points down and one load points up.

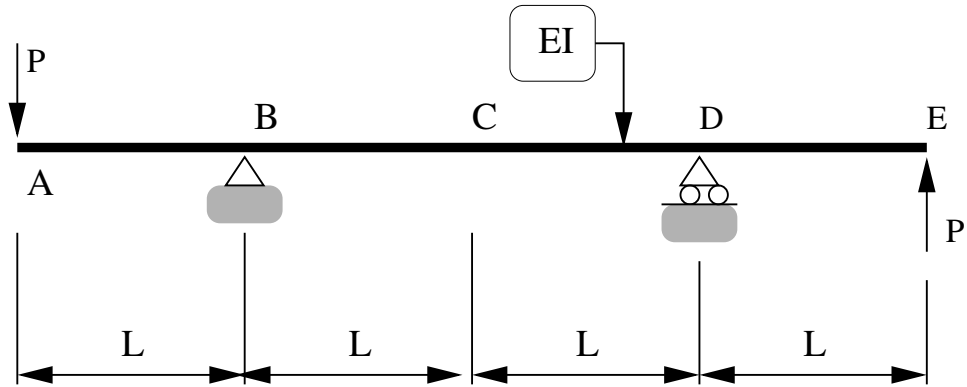


Figure 2: Simple beam structure with skew-symmetric loading pattern.

[1e] (4 pts) Use the method of **virtual forces** and a coordinate system positioned at C to show that the vertical displacement of C is zero, i.e.,  $\Delta_C = 0$ .

Now consider the problem.

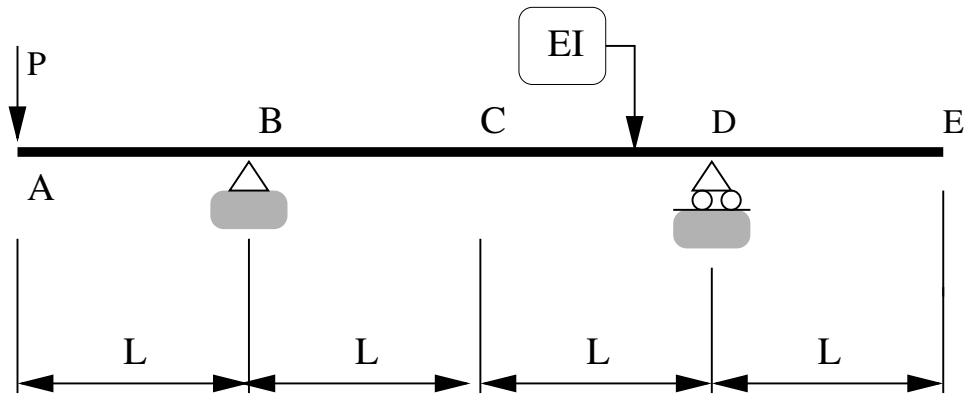


Figure 3: Simple beam structure with external load  $P$ .

[1f] (3 pts) Use your answers from parts [1b] and [1e] to write down an expression for the vertical deflection at C due to the loading pattern shown in Figure 3.

Note: You should find this is a one line answer.

**Question 2: 10 points**

**OPTIONAL: Derive Elastic Curve for Beam Deflection.** Figure 5 is a front elevation view of a cantilevered beam carrying a single point load  $P$ .  $EI$  is constant along the beam structure A-B-C-D.

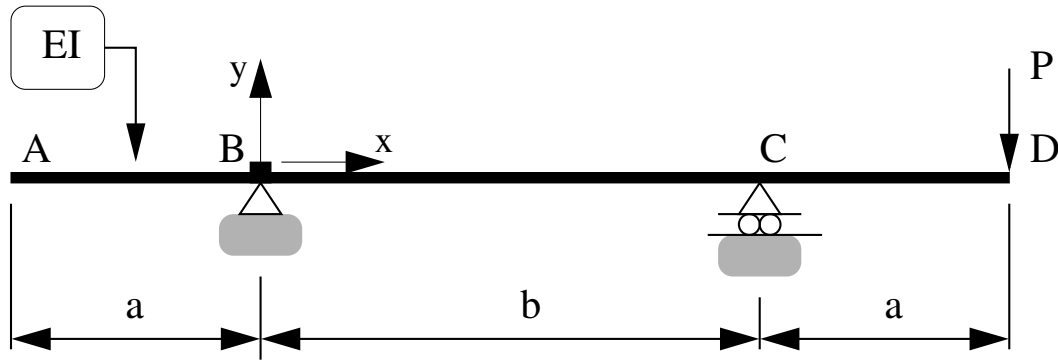


Figure 4: Cantilevered beam carrying a single applied load  $P$ .

**[2a]** (5 pts) Starting from the differential equation,

$$\frac{d^2y}{dx^2} = \left[ \frac{-M(x)}{EI} \right], \quad (5)$$

(notice the minus sign on  $M(x)$ ) and appropriate boundary conditions, show that that rotation of the beam at C is:

$$\theta(b) = \frac{-Pab}{3EI}. \quad (6)$$

Now let's move the x-y coordinate system to point C, i.e.,

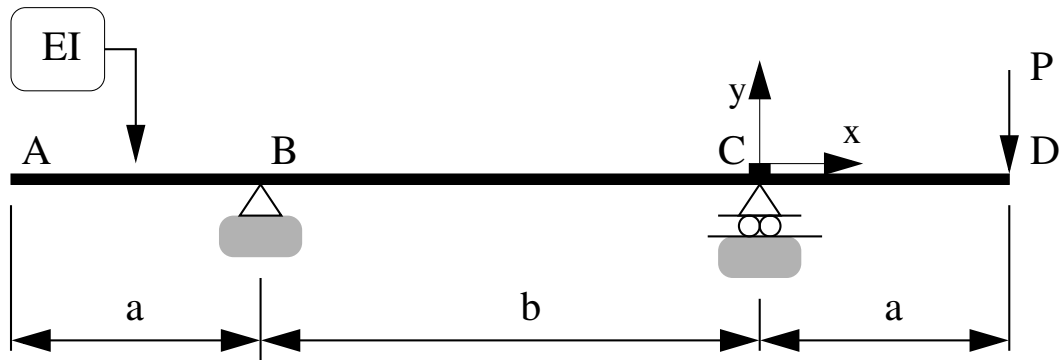


Figure 5: Cantilevered beam carrying a single applied load P.

[2b] (5 pts) Starting from the differential equation,

$$\frac{d^2y}{dx^2} = \left[ \frac{-M(x)}{EI} \right], \quad (7)$$

in the new coordinate system, show that that vertical deflection (measured downwards) at D is:

$$y_D = \frac{Pa^2}{3EI} (a + b). \quad (8)$$

Show all of your working.

**Question 3: 10 points**

**OPTIONAL: Computing Displacements with the Method of Virtual Forces.** Figure 6 is a front elevation view of a dog-leg cantilever beam carrying a point load  $P$  (N) at point D.

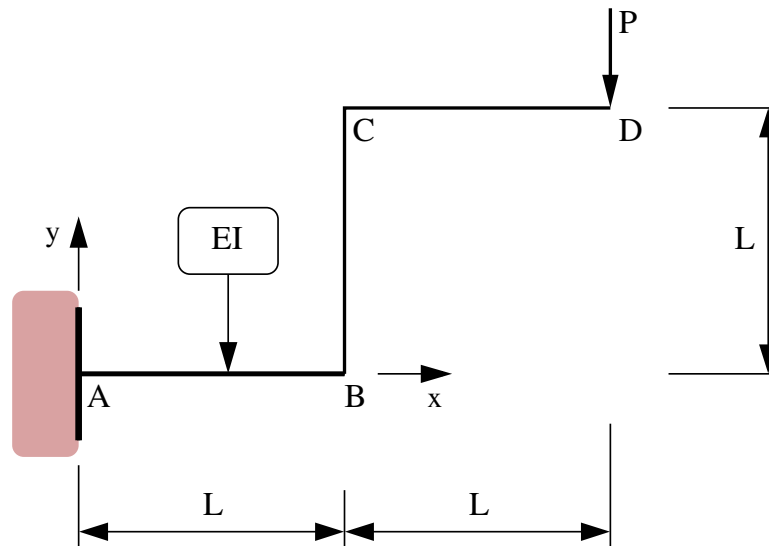


Figure 6: Dog-leg cantilever beam carrying end moment  $M$  (N.m).

The flexural stiffness  $EI$  is constant along A-B-C-D. The axial stiffness  $EA$  is very high and, as such, axial displacements can be ignored in the analysis.

**[3a]** (4 pts) Use the method of **virtual forces** to show that the clockwise rotation of the beam at point D is:

$$\theta_d = \frac{3PL^2}{EI}. \quad (9)$$



**[3b]** (6 pts) Use the method of **virtual forces** to show that the horizontal (measured left-to-right) and vertical (measured downwards) displacements at D are:

$$\Delta_h = \frac{2PL^3}{EI}. \quad (10)$$

and

$$\Delta_v = \frac{11}{3} \frac{PL^3}{EI}. \quad (11)$$

respectively. Show all of your working.

Question 3b continued ...

**Question 4: 10 points**

**OPTIONAL: Bending Moment and Curvature in an Elastic Beam.** Figure 7 is a front elevation view of a simply supported beam that carries a trapezoid load.

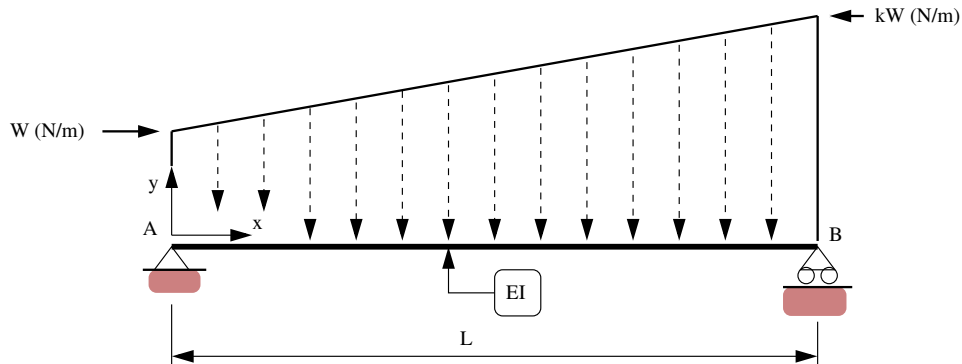


Figure 7: Simply supported beam carrying a trapezoid load.

The load increases from  $W$  (N/m) at  $x = 0$ , to  $kW$  (N/m) at  $x = L$ , where  $k$  is a non-negative constant. Thus, the total beam loading is  $\frac{WL}{2} (1 + k)$ .

**[4a]** (2 pts). Starting from first principles of engineering, show that the vertical reactions at A and B are:

$$V_A = \frac{WL}{6} (2 + k) \quad \text{and} \quad V_B = \frac{WL}{6} (1 + 2k). \quad (12)$$

**[4b]** (3 pts). Show that the bending moment at point  $x$  is:

$$M(x) = \frac{WL^2}{6} \left( \frac{x}{L} \right) \left[ (2 + k) - 3 \left( \frac{x}{L} \right) + (1 - k) \left( \frac{x}{L} \right)^2 \right]. \quad (13)$$

Notice that  $M(0) = M(L) = 0$ , regardless of the value of  $k$ .

The math for this part is a bit tedious – hence, I suggest you work out a solution on a separate sheet of paper, then write a tidy solution here.

**[4c]** (3 pts). Hence, show that the location of maximum curvature  $\phi$  in the beam corresponds to the solution of the quadratic equation:

$$3(1 - k)x^2 - 6Lx + (2 + k)L^2 = 0. \quad (14)$$

**[4d]** (2 pts). For the case where  $k = 1$  (i.e., a constant uniform loading), use equations 13 and 14 to determine the position and value of the maximum bending moment.

**Question 5: 10 points**

**OPTIONAL: Use Principle of Virtual Work to Compute Displacements.** Consider the articulated cantilever beam structure shown in Figure 8.

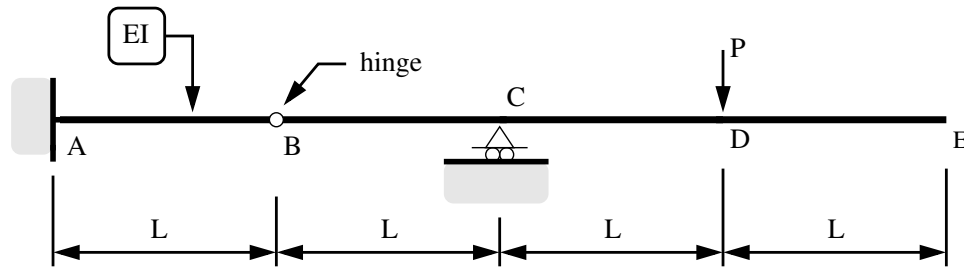


Figure 8: Elevation view of articulated cantilever beam structure.

At Point A, the cantilever is fully fixed (no movement) to a wall. Point B is a hinge. Both members have cross section properties  $EI$ . A single point load  $P$  (N) is applied at node D as shown in the figure.

**[5a]** (2 pts). Draw and label the bending moment diagram for this problem.

**[5b]** (2 pts). Qualitatively sketch the deflected shape. Indicate regions of tension/compression, and any points where slope of the beam is discontinuous.

[5c] (6 pts). Use the method of **virtual forces** to compute the **vertical displacement** and **end rotation** of the beam at E.

Show all of your working.

Question [5c] continued:



**Question 6: 10 points**

**OPTIONAL: Zero-Force Members, Virtual Work, and Flexibility Matrices.** Consider the truss structure shown in Figure 9.

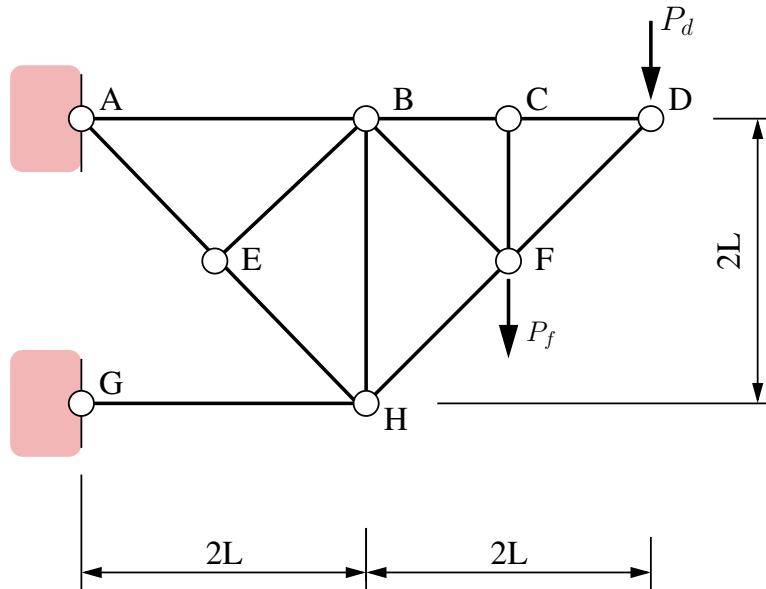


Figure 9: Elevation view of a pin-jointed truss.

The horizontal and vertical degrees of freedom are fully-fixed at supports A and G. The truss carries vertical loads  $P_d$  and  $P_f$  at nodes D and F, respectively. All frame members have cross section properties  $AE$ .

**[6a]** (3 pts) Use the method of **virtual forces** to compute the vertical deflection at node D due to load  $P_d$  alone (i.e.,  $P_f = 0$ ).

Question [6a] continued:

**[6b]** (3 pts) Use the method of **virtual forces** to compute the vertical deflection at node F due to load  $P_f$  alone (i.e.,  $P_d = 0$ ).

**[6c]** (4 pts) Use the method of **virtual forces** to compute the two-by-two flexibility matrix connecting the vertical displacements at points D and F to applied loads  $P_d$  and  $P_f$ , i.e., as a function of  $P_d$ ,  $P_f$ ,  $L$  and  $AE$ .

$$\begin{bmatrix} \Delta_d \\ \Delta_f \end{bmatrix} = \begin{bmatrix} f_{11} & f_{12} \\ f_{21} & f_{22} \end{bmatrix} \begin{bmatrix} P_d \\ P_f \end{bmatrix}. \quad (15)$$